

# LOCATION RESTRICTIONS DEMONSTRATION REPORT UPPER AQC IMPOUNDMENT

## La Cygne Generating Station

Presented to:  
Kansas City Power & Light Company  
La Cygne Generating Station  
La Cygne, Kansas

**SCS ENGINEERS**

27218131.04 | October 2018

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# 1 INTRODUCTION AND PURPOSE

The Disposal of Coal Combustion Residuals (CCR) from Electric Utilities Final Rule (CCR Rule) 40 CFR 257.60 through 257.64 requires owner/operators of existing CCR units to make demonstrations in the event a unit is located in certain areas. The purpose of this report is to demonstrate whether the Upper AQC Impoundment (Unit) at Kansas City Power & Light Company's (KCP&L) La Cygne Generating Station (La Cygne) is located in any of those areas; and, if so, to make certain demonstrations per the CCR Rule that will permit continued CCR disposal/management operations.

The Unit, which is an existing CCR surface impoundment, is located at the La Cygne Generating Station near La Cygne, Kansas, as indicated in **Figure 1**.

SCS Engineers (SCS) has reviewed the documents provided in Section 7 and completed site visit(s) to develop this report. This document provides demonstrations that documents if the Unit is located:

- with a base that is constructed no less than 5 feet above the upper limit of the uppermost aquifer (40 CFR §257.60);
- in wetlands (40 CFR §257.61);
- within 200 feet of the outermost damage zone of a fault which has been displaced in Holocene time (40 CFR §257.62);
- within a seismic impact zone (40 CFR §257.63); and
- in an unstable area (40 CFR §257.64).

The applicable CCR Rule requirement for each of the above is listed in the respective section in italics followed by an explanation of the review and determinations completed by SCS.

## 2 PLACEMENT ABOVE THE UPPERMOST AQUIFER (§257.60)

*§257.60 (a) New CCR landfills, existing and new CCR surface impoundments, and all lateral expansions of CCR units must be constructed with a base that is located no less than 1.52 meters (five feet) above the upper limit of the uppermost aquifer, or must demonstrate that there will not be an intermittent, recurring, or sustained hydraulic connection between any portion of the base of the CCR unit and the uppermost aquifer due to normal fluctuations in groundwater elevations (including the seasonal high water table). The owner or operator must demonstrate by the dates specified in paragraph (c) of this section that the CCR unit meets the minimum requirements for placement above the uppermost aquifer.*

SCS compared the location and elevation of the base of the Unit (approximately 872 feet MSL [mean sea level] to the east to 846 feet MSL to the west) to the elevation of the upper limit of the uppermost aquifer by reviewing the site geology as characterized by AECOM in the Detailed Hydrogeologic Site Characterization Report (DSI) prepared in October 2017 (AECOM, 2017). Pertinent sections of this report have been provided in **Appendix A** summarizing and showing the location of the base of the Unit and the uppermost aquifer. As described in the investigation, the generalized geology underlying the Unit includes the following, from the surface down:

1. Native residual clay with some discrete sand beds or lenses (semi-confining to confining unit)
2. Unsaturated or relatively low-yielding shale (semi-confining to confining unit)
3. Saturated unweathered to highly weathered shale (Holdenville Shale) with relatively higher permeability (uppermost aquifer)
4. Relatively unweathered lower permeability shale with sparse limestone and coal units interbedded (lower confining bedrock unit)

The site investigation completed by AECOM confirmed that unconsolidated deposits consisted of clay fill and residual clay material with discrete sand beds or lenses from ground surface to approximately 1 to 31 feet below ground surface (bgs). Clay fill and residual clay material was encountered around the perimeter of the Upper AQC Impoundment. Unsaturated or relatively low-yielding shale was encountered below the unconsolidated residual clayey material. This low-yielding primarily shale material transitioned into a saturated relatively higher permeability unweathered to highly weathered zone which is defined as the uppermost aquifer beneath the impoundment. The thickness of the aquifer is believed to be roughly 5 to 10 feet thick with upper elevations ranging from approximately 820 feet MSL near the west edge of the impoundment to 850 feet MSL near the east edge of the impoundment.

The aquifer appears locally confined or semi-confined by relatively lower permeability shale bedrock and residual clay acting as an upper confining unit, and a relatively lower permeability primarily shale bedrock on the bottom. Although groundwater levels measured in the wells may occasionally extend up and into the low permeability shale and clay, the groundwater level is believed to be representative of the potentiometric head and not the water table elevation.

Based on review of the historic topographic map of the area prior to the station being constructed, the base of the impoundment appears to range from approximately 872 feet MSL to the east to 846 feet MSL to the west. A review of hydrostratigraphic cross sections and boring logs in the report indicate the maximum uppermost aquifer elevation beneath the impoundment is approximately 850 feet MSL near the east edge of the impoundment and approximately 820 feet MSL near the west edge of the impoundment. Based on this review, the base of the Unit appears to be approximately 22 feet above the upper limit of the uppermost

## Location Restrictions Demonstration Report

aquifer, therefore the base of the Unit was constructed no less than five feet above the upper limit of the uppermost aquifer. Consequently, no additional demonstration is necessary.

### 3 WETLANDS (§257.61)

*§257.61 (a) New CCR landfills, existing and new CCR surface impoundments, and all lateral expansions of CCR units must not be located in wetlands, as defined in §232.2 of this chapter, unless the owner or operator demonstrates by the dates specified in paragraph (c) of this section that the CCR unit meets the requirements of paragraphs (a)(1) through (5) of this section.”*

A figure developed for this analysis is provided as **Figure 2**. A Certified Wetland Delineator with SCS visited the Unit on May 2, 2018 to determine if any areas within the boundaries of the Unit are potentially located in existing wetland areas as defined in 40 CFR §232.2. The areas reviewed are indicated on **Figure 2**. Based on this review, SCS determined the Unit is not located within a wetland area, as defined in 40 CFR §232.2. Consequently, no additional demonstration is necessary.

## 4 FAULT AREAS (§257.62)

*§257.62 (a) New CCR landfills, existing and new CCR surface impoundments, and all lateral expansions of CCR units must not be located within 60 meters (200 feet) of the outermost damage zone of a fault that has had displacement in Holocene time unless the owner or operator demonstrates by the dates specified in paragraph (c) of this section that an alternative setback distance of less than 60 meters (200 feet) will prevent damage to the structural integrity of the CCR unit.*

SCS compared the location of the Unit to the location of faults as shown in the United States Geologic Survey (USGS) Quaternary Faults and Folds Database for the United States. The nearest fault area is indicated on **Figure 3**. Based on this review, SCS determined the Unit is not located within 200 feet of the outermost damage zone of a fault that has had displacement in the Holocene time, the most recent portion of the Quaternary Age. Consequently, no additional demonstration is necessary.

## 5 SEISMIC IMPACT ZONES (§257.63)

*§257.63 (a) New CCR landfills, existing and new CCR surface impoundments, and all lateral expansions of CCR units must not be located in seismic impact zones unless the owner or operator demonstrates by the dates specified in paragraph (c) of this section that all structural components including liners, leachate collection and removal systems, and surface water control systems, are designed to resist the maximum horizontal acceleration in lithified earth material for the site.*

SCS compared the location of the Unit to the location of seismic impact zones as defined in §257.53, as shown in the USGS map “Two Percent Probability of Exceedance in 50 Years Map of Peak Ground Acceleration”. The location of the Unit in relation to the nearest seismic impact zones (i.e., in areas of at least 0.1 g as shown on the map in dark blue) are indicated on **Figure 4**. The Unit falls within the 0.04 g to 0.06g range of the map. Based on this review, SCS determined the Unit is not located within a seismic impact zone. Consequently, no additional demonstration is necessary.

## 6 UNSTABLE AREAS (§257.64)

**§257.64 (a)** *An existing or new CCR landfill, existing or new CCR surface impoundment, or any lateral expansion of a CCR unit must not be located in an unstable area unless the owner or operator demonstrates by the dates specified in paragraph (d) of this section that recognized and generally accepted good engineering practices have been incorporated into the design of the CCR unit to ensure that the integrity of the structural components of the CCR unit will not be disrupted.*

SCS evaluated the location of the Unit for the presence of on-site or local unstable areas as defined in §257.53. Evaluations of the conditions listed in §257.64 (b)(1) through (3) were evaluated and are discussed below. Based on this review, SCS determined the Unit is not located within an unstable area as defined in §257.53. Consequently, no additional demonstration is necessary.

**257.64 (b)** *The owner or operator must consider all of the following factors, at a minimum, when determining whether an area is unstable:*

### 6.1 UNSTABLE FACTORS CONSIDERED: DIFFERENTIAL SETTLING (§257.64(b)(1))

*On-site or local soil conditions that may result in significant differential settling;*

SCS has visited the site and evaluated site-specific reports detailing the conditions of the on-site and local soils for conditions that could result in significant differential settling. The Unit was characterized in the “Detailed Hydrogeologic Site Characterization Report, Upper AQC Impoundment” (DSI) prepared by AECOM in October 2017 together with “Geotechnical Data for Bidders, New FGD Sludge Retention Dam – Stage 1” (FGD Geotechnical Report), prepared by Woodward-Clyde Consultants in December 1978 (WCC, 1978). The Unit is located within the Osage Cuestas physiographic section (Kansas Geological Survey) of eastern Kansas. Soils in the area are generally residual fine grained soils overlying shale, limestone, and sandstone.

According to the DSI (AECOM, 2017) and the FGD Geotechnical Report (WCC, 1978), a varying thickness of natural soil and fill materials underlie the Unit. The natural soils vary from brown to gray, low to highly plastic clays (CH), silty clays (CL) and sandy clays (SC) with some discrete poorly graded sand beds or lenses (SP) as defined by the Unified Soil Classification System (USCS). The majority of the soils are residual deposits formed from the weathering of the underlying bedrock, although some alluvial or colluvial soils may be present. These unconsolidated deposits vary in thickness from 2 to 20 feet across the Unit location.

The bedrock near the Unit consists of shale, limestone, sandstone and minor coal, assigned to the Middle to Upper Pennsylvanian Age, Marmaton and Pleasanton Groups (KGS, 1969). These units are relatively typical of the cyclic sedimentary rock deposits present in the Midwest.

Based on the geologic description above and a review of geotechnical data in the report(s), it is SCS’ professional opinion that the soils on site will not experience significant differential settlement. Pertinent sections of the 2017 AECOM report are provided in **Appendix B.1** describing the soils at and near the Unit. Based on this review, SCS determined the Unit is not located within an area with on-site or local soil conditions that may result in significant differential settling. Additional demonstration(s) are not required.

## 6.2 UNSTABLE FACTORS CONSIDERED: GEOLOGIC/GEOMORPHOLOGIC FEATURES (§257.64(b)(2))

*On-site or local geologic or geomorphologic features; and*

SCS has visited the Unit and evaluated published data and site-specific reports for the presence of on-site or local geologic and geomorphologic features, to include karst terrain, steep slopes, and sinkholes. Documents and websites reviewed include:

- KGS Earthquakes and Hazards (<http://geokansas.ku.edu/sinkholes>)
- Land Subsidence in Central Kansas Related to Salt Dissolution (<http://www.kgs.ku.edu/Publications/Bulletins/214/index.html>)
- Appendix D—Surface Sinkholes and Other Solution Features, Geologic History of Kansas ([http://www.kgs.ku.edu/Publications/Bulletins/162/10\\_app\\_d.html](http://www.kgs.ku.edu/Publications/Bulletins/162/10_app_d.html))
- Digital engineering Aspects of Karst Map, Tobin and Weary, USGS Open-File Report 2004-1352 ([pubs.usgs.gov/of/2004/1352/data/USA\\_karst.pdf](http://pubs.usgs.gov/of/2004/1352/data/USA_karst.pdf))

Neither the Kansas Geologic Survey website nor published data indicate the presence of karst terrain, sinkholes, caves, or ground conditions that could cause a structural failure in the area of the Unit or region around the Unit. Available data indicates most sinkholes in Kansas are related to salt dissolution (central part of the state) and caves, natural bridges, underground drainage, and soil cracks that are located predominately in the western part of the state. Sinkholes related to human-made features are discussed in Section 6.3

SCS' visits to the Unit and a review of terrain at and near the Unit indicated no steep slopes, terrain features, or other local geologic or geomorphologic features that could feasibly result in an unstable condition. Pertinent documents and sections of documents reviewed are provided in Appendix B.2, and indicate the location of the Unit in relation to the known geologic or geomorphologic features nearest the Unit.

Based on this review, SCS determined the Unit is not located within an area with on-site or local geologic or geomorphologic features that would result in an unstable environment for the Unit. Additional demonstration(s) are not required.

### 6.3 UNSTABLE FACTORS CONSIDERED: HUMAN-MADE FEATURES OR EVENTS (§257.64(b)(3))

*On-site or local human-made features or events (both surface and subsurface).*

SCS has visited the Unit and evaluated published data and site-specific reports for the presence of on-site or local human-made features or events (both surface and subsurface), to include surface and subsurface mining, extensive withdrawal of oil and gas, steep slopes, and sources of rapid groundwater drawdown, in strata that could feasibly impact the Unit. Documents and websites reviewed include:

- AECOM (2017), Detailed Hydrogeologic Site Characterization Report, Upper AQC Impoundment, La Cygne Generating Station.
- Woodward Clyde Consultants, Geotechnical Data for Bidders, New FGD Sludge Retention Dam – Stage 1, Volume 2, La Cygne Steam Electric Station, December 1978.
- Coal Resources of the Marmaton Group in Eastern Kansas, Walter H. Schoewe, 1955, Kansas Geological Survey Bulletin 114, Part 2.
- Kansas Mines (<http://www.miningartifacts.org/Kansas-Mines.html>)
- Interstate Technical Group on Abandoned Underground Mines Third Biennial Workshop, Mining History in Kansas, Lawrence L. Brady, KGS (<https://www.fhwa.dot.gov/engineering/geotech/hazards/mine/workshops/kdot/kansas01.cfm#fig4>)
- Coal Mining (<http://geokansas.ku.edu/coal-mining>)
- Mining in Kansas, Kansas Historical Society (<https://www.kshs.org/index.php?url=kansapedia/mining-in-kansas/15603>)
- Google Earth Aerial photographs

SCS reviewed the above historical records related to coal mining in the immediate vicinity of the Unit. Subsurface mining appears to have occurred on station property prior to the development of the generating station. Based on coal production data, the mines in the area are believed to be small and to have only served local trade (KGS, 1955). The exact location of each mine relative to the footprint of the Lower AQC cannot be determined from the KGS-provided information, some of which is dated before 1900; however, no mines were indicated in Section 27, Township 19 South, Range 25 East, where the Unit is located

SCS' review of the aerial photographs around the generating station showed indications of strip mining to the southeast, ranging from approximately 2.6 miles to 6 miles from the Unit. The aerial photographs confirm the presence of strip mining in the area; however none of the borings drilled for the ash impoundments or adjacent landfill reviewed by SCS indicated the presence of a heterogeneous backfill associated with strip mining.

SCS' internet search located a 1940 bulletin from the KGS titled "Oil and Gas in Linn County, Kansas". Plate 3 of Bulletin 30 is an Oil and Gas Exploration Map of Linn County. The map shows locations of wells drilled, various dry holes, oil wells, and oil wells with gas. No exploration or well drilling in the sections encompassing the generating station is indicated on the map.

## Location Restrictions Demonstration Report

No evidence of steep slopes in the vicinity of the unit nor areas of rapid groundwater drawdown were identified.

Pertinent documents and sections of documents reviewed include soil boring data for La Cygne (Ebasco, 1969) and a table with the location of nearby coal mines (Schoewe, 1955), provided in **Appendix B.3**.

Based on this review, SCS determined the Unit is not located within an area with on-site or local human-made features or events (both surface and subsurface) that could feasibly result in an unstable condition at the Unit. Additional demonstration(s) are not required.

### 7 REFERENCES

AECOM (2017), Detailed Hydrogeologic Site Characterization Report, CCR Landfill & Lower AQC Impoundment, La Cygne Generating Station.

AECOM (2017), Detailed Hydrogeologic Site Characterization Report, Upper AQC Impoundment, La Cygne Generating Station.

Ebasco Services Incorporated (1969), Foundation Investigation Plan Set.

Geotechnology, Inc. (2009), Drilling, Laboratory, Geophysical, and Pressuremeter Results, Environmental Retrofit Project, La Cygne Generating Station La Cygne, Kansas.

Google Earth (2014), Aerial Photograph dated 10/20/2014.

Jewett, J.M., O'Connor, H.G., and Zeller, D.E., (1968), Stratigraphic Succession in Kansas – Pennsylvanian System. Kansas Geological Survey Bulletin 189.

Jewett, J.M., with Lee, Wallace and Keroher, R.P., (1940), Oil and Gas in Linn County, Kansas. Kansas Geological Survey Bulletin 30.

Kansas Geological Survey, (1969), Geologic Map of Linn County, Kansas.

Kansas Geological Society (1893), State Inspector of Coal Mines, Sixth Annual Report

Kansas State Labor Department (1950-52), Mine Inspection Division Report.

Physiographic Map of Kansas, <http://www.kgs.ku.edu/Physio/physio.html>.

Schoewe, Halter H., (1955), Coal Resources of the Marmaton Group in Eastern Kansas, Kansas Geological Survey Bulletin 114, Part 2.

Woodward Clyde Consultants, (1978), Geotechnical Data for Bidders, New FGD Sludge Retention Dam – Stage 1, Volume 2, La Cygne Steam Electric Station.

USGS Maps: La Cygne, USGS US Topo 7.5 (1958) KS\_La Cygne\_511759\_1958\_24000\_geo

Boicourt, USGS US Topo 7.5 (2015) KS\_Boicourt\_20151230\_TM\_geo

Boicourt, USGS US Topo 7.5 (1986) KS\_Boicourt\_801573\_1986\_24000\_geo

## 8 QUALIFIED PROFESSIONAL ENGINEER CERTIFICATION (§§257.60(B), 257.61(B), 257.62(B), 257.63(B), 257.64(C))

The undersigned registered professional engineer is familiar with the requirements of the CCR Rule and has visited and examined the Unit and/or has supervised examination of the Unit and development of this report by appropriately qualified personnel. I hereby certify based on a review of available information and observations, that this report meets the requirements of paragraphs §§257.60(a), 257.61(a), 257.62(a), 257.63(a) and 257.64(a).

Professional Engineer: Company: \_\_\_\_\_ Douglas L. Doerr, P.E. \_\_\_\_\_

\_\_\_\_\_ SCS Engineers \_\_\_\_\_

PE Registration State: \_\_\_\_\_ Kansas \_\_\_\_\_

Registration Number: \_\_\_\_\_ 14136 \_\_\_\_\_

Professional Engineer Seal:



## Location Restrictions Demonstration Report

The undersigned registered professional geologist is familiar with the requirements of the CCR Rule and has visited and examined the Unit and/or has supervised examination of the Unit and development of this report by appropriately qualified personnel. I hereby certify based on a review of available information and observations, that this report meets the requirements of paragraphs §§257.60(a).

Professional Geologist: Company: \_\_\_\_\_ John R. Rockhold, P.G. \_\_\_\_\_

\_\_\_\_\_ SCS Engineers \_\_\_\_\_

PG Registration State: \_\_\_\_\_ Kansas \_\_\_\_\_

Registration Number: \_\_\_\_\_ 0029 \_\_\_\_\_

Professional Geologist Seal:



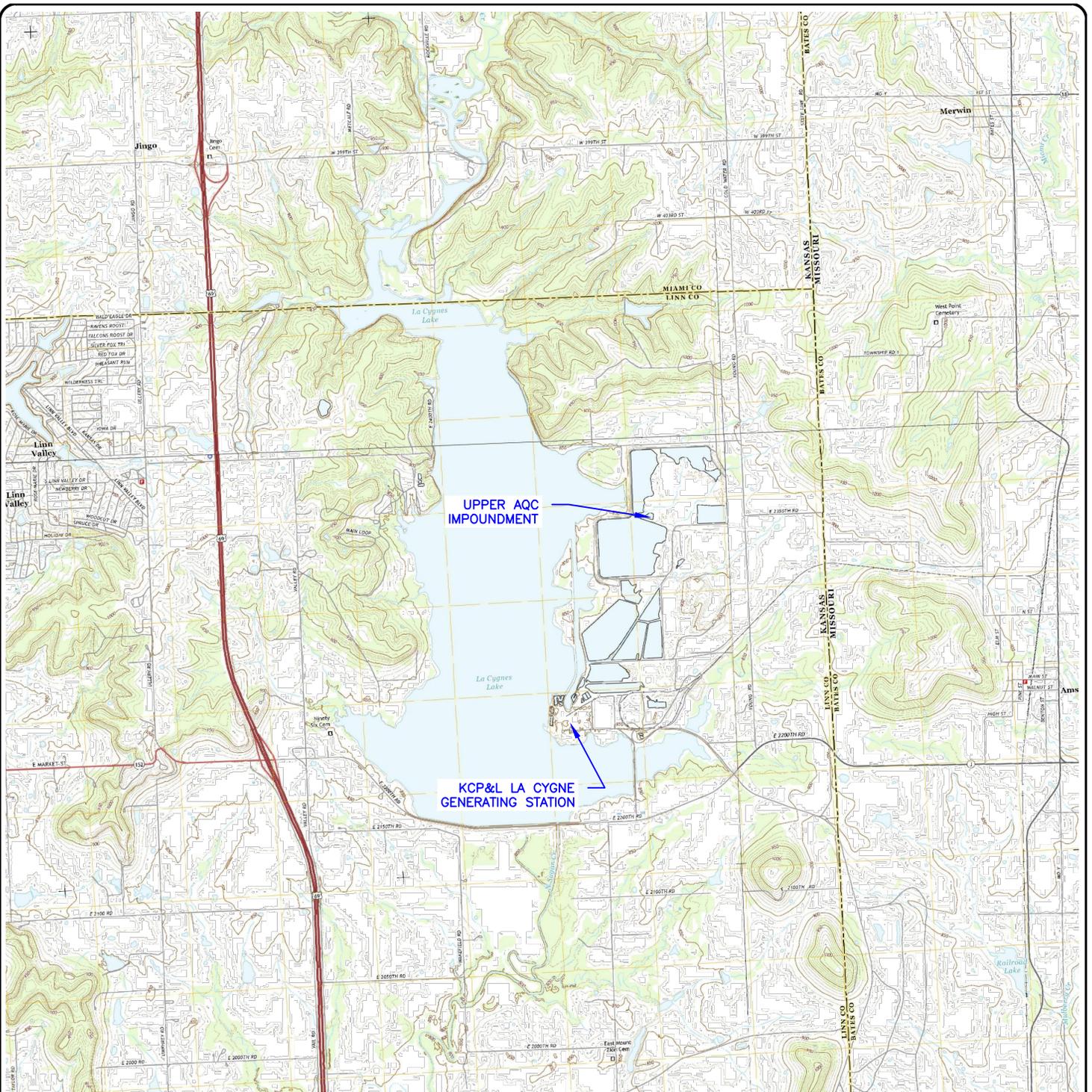
## FIGURES

Figure 1 - Site Location Map

Figure 2 - Wetlands Map

Figure 3 - Fault Areas Map

Figure 4 - Horizontal Acceleration Map



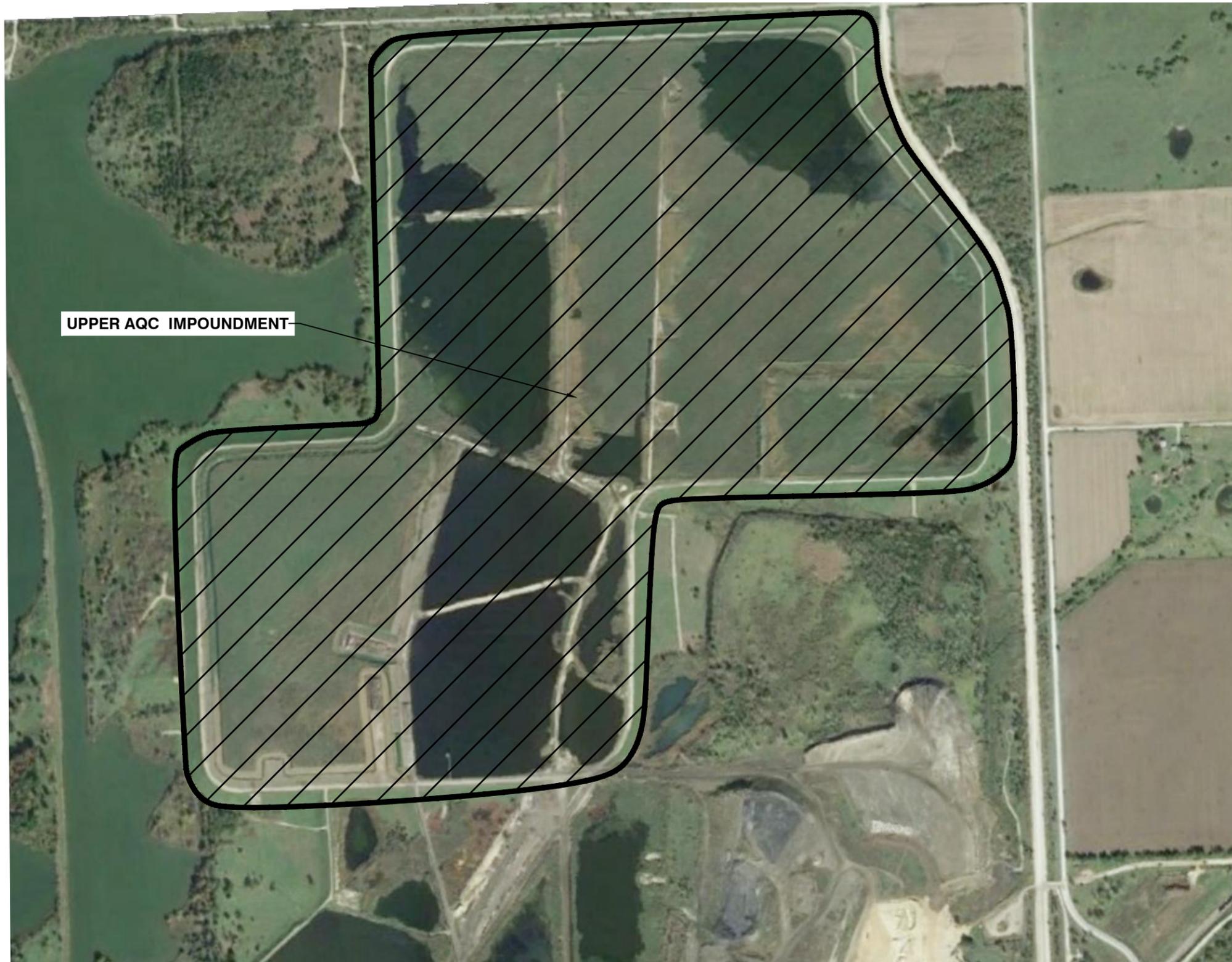
SOURCE:  
2015 USGS MAP  
NEW LANCASTER-KS, BOICOURT-KS,  
AMORET-MO, AND DREXEL-MO  
7.5 MINUTE SERIES (TOPOGRAPHIC)

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### FIGURE 1 SITE LOCATION MAP KCP&L LA CYGNE GENERATING STATION - UPPER AQC IMPOUNDMENT LA CYGNE, KANSAS

CHK. BY: DLD	DWN. BY: TGW	DSN. BY: TGW	PROJ. NO. 27218131.04
PROJ. MGR: DLD	DATE: 10/2018	CADD FILE: FIGURE 1_SITE MAP.DWG	DRAWING NO. 1



UPPER AQC IMPOUNDMENT

**LEGEND:**



AREAS THAT WERE FOUND NOT TO INCLUDE WETLANDS BASED ON REVIEW OF VEGETATION, SOIL TYPE, AND HYDROLOGIC CHARACTERISTICS.

**NOTES:**

1. GOOGLE EARTH IMAGE DATED OCTOBER 2014.



**FIGURE 2  
WETLANDS MAP**

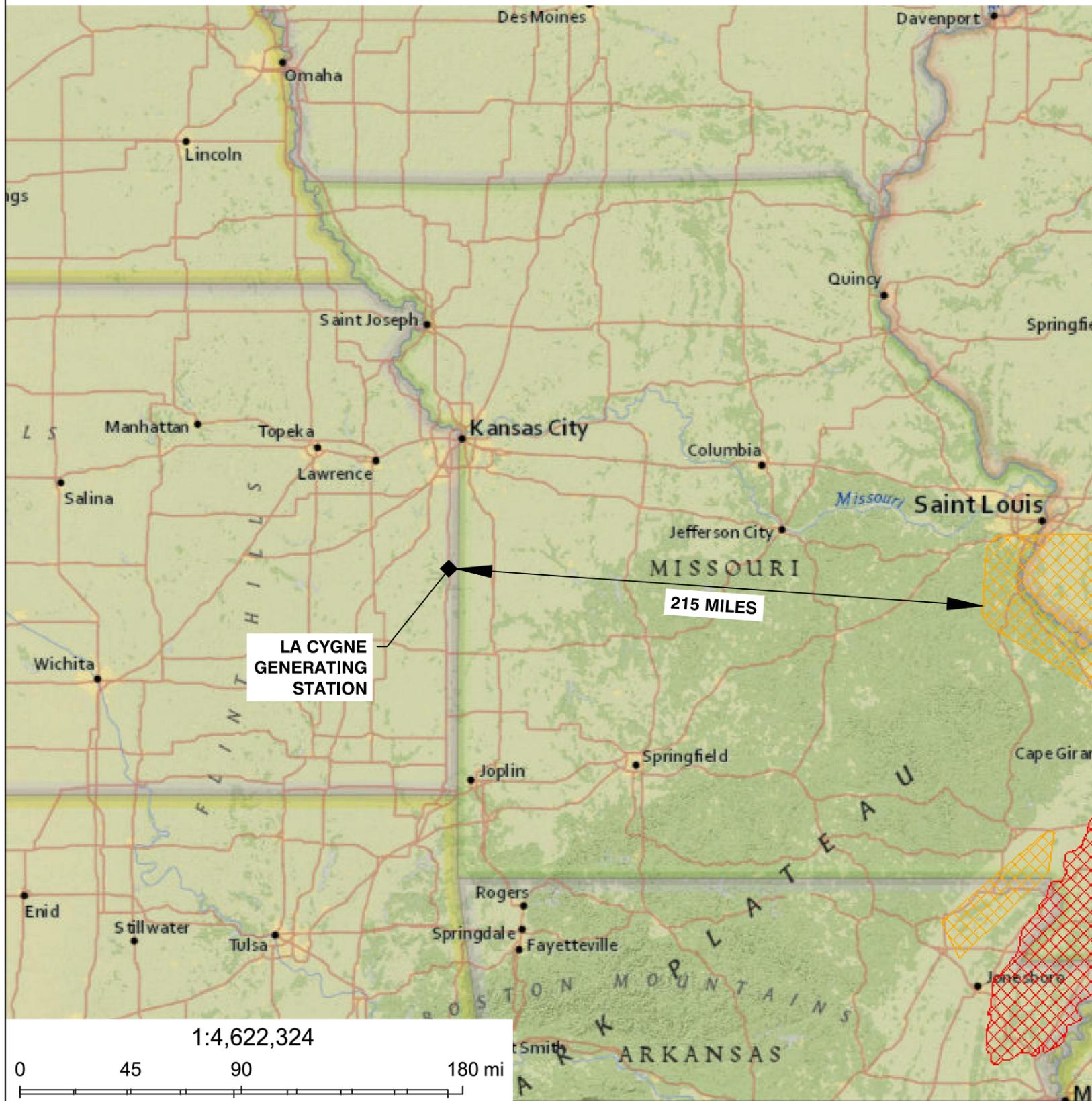
KCP&L LA CYGNE GENERATING STATION - UPPER AQC IMPOUNDMENT  
LA CYGNE, KANSAS

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CHK. BY: DLD	DWN. BY: TGW	DSN. BY: TGW	PROJ. NO. 27218131.04
PROJ. MGR: DLD	DATE: 10/2018	CADD FILE: FIGURE 2_WETLANDS.DWG	

# USGS Quaternary Faults and Folds Database



- ▲ Site Investigations
- Quaternary Faults
  - historical (<150 years), well constrained location
  - - - historical (<150 years), moderately constrained location
  - · · historical (<150 years), inferred location
  - latest Quaternary (<15,000 years), well constrained location

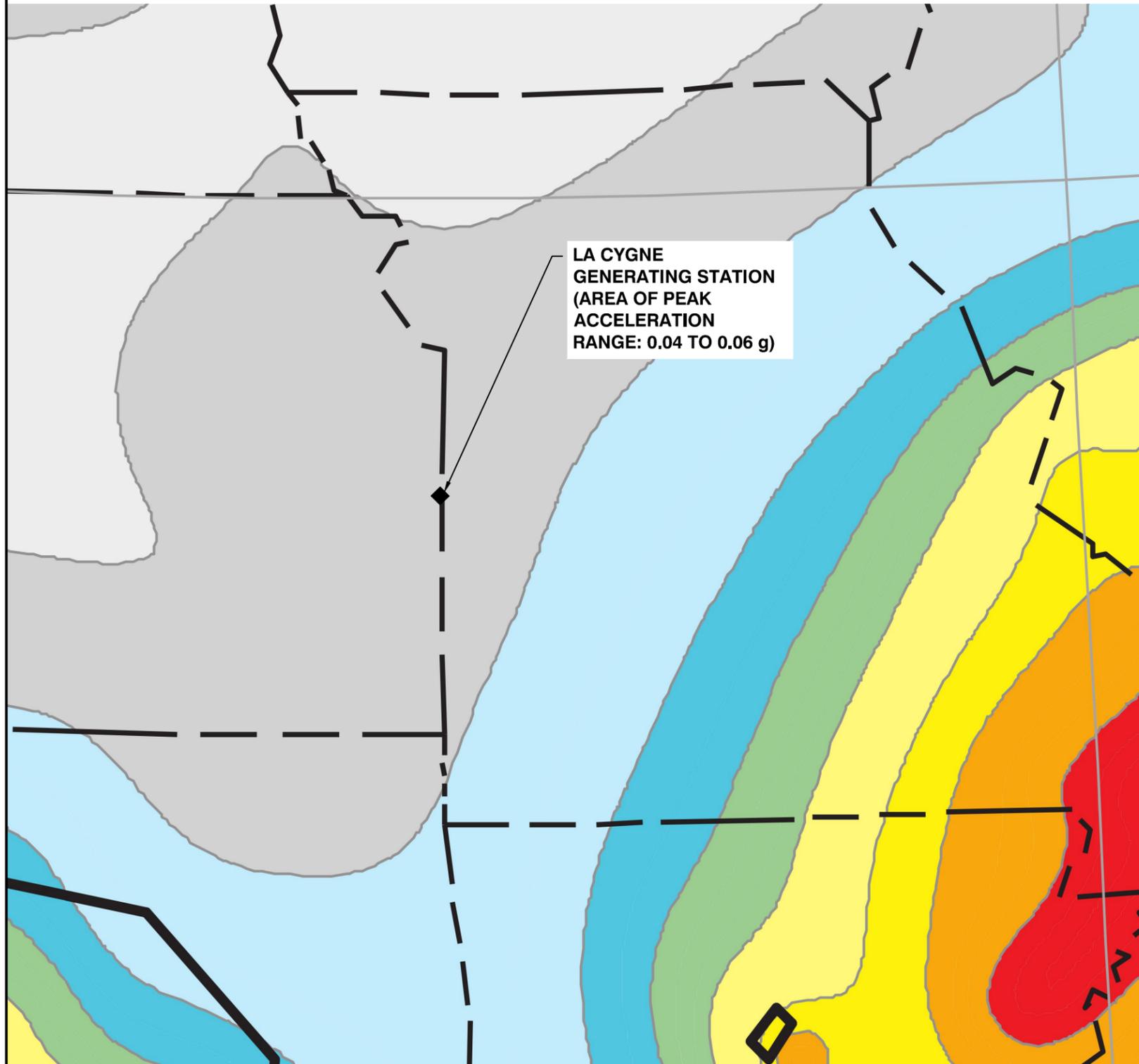
Content may not reflect National Geographic's current map policy.  
 Sources: National Geographic, Esri, Garmin, HERE, UNEP-WCMC, USGS, NASA, ESA, METI, NRCAN, GEBCO, NOAA, increment P Corp.

Source: USGS fault map - <https://earthquake.usgs.gov/hazards/qfaults/>



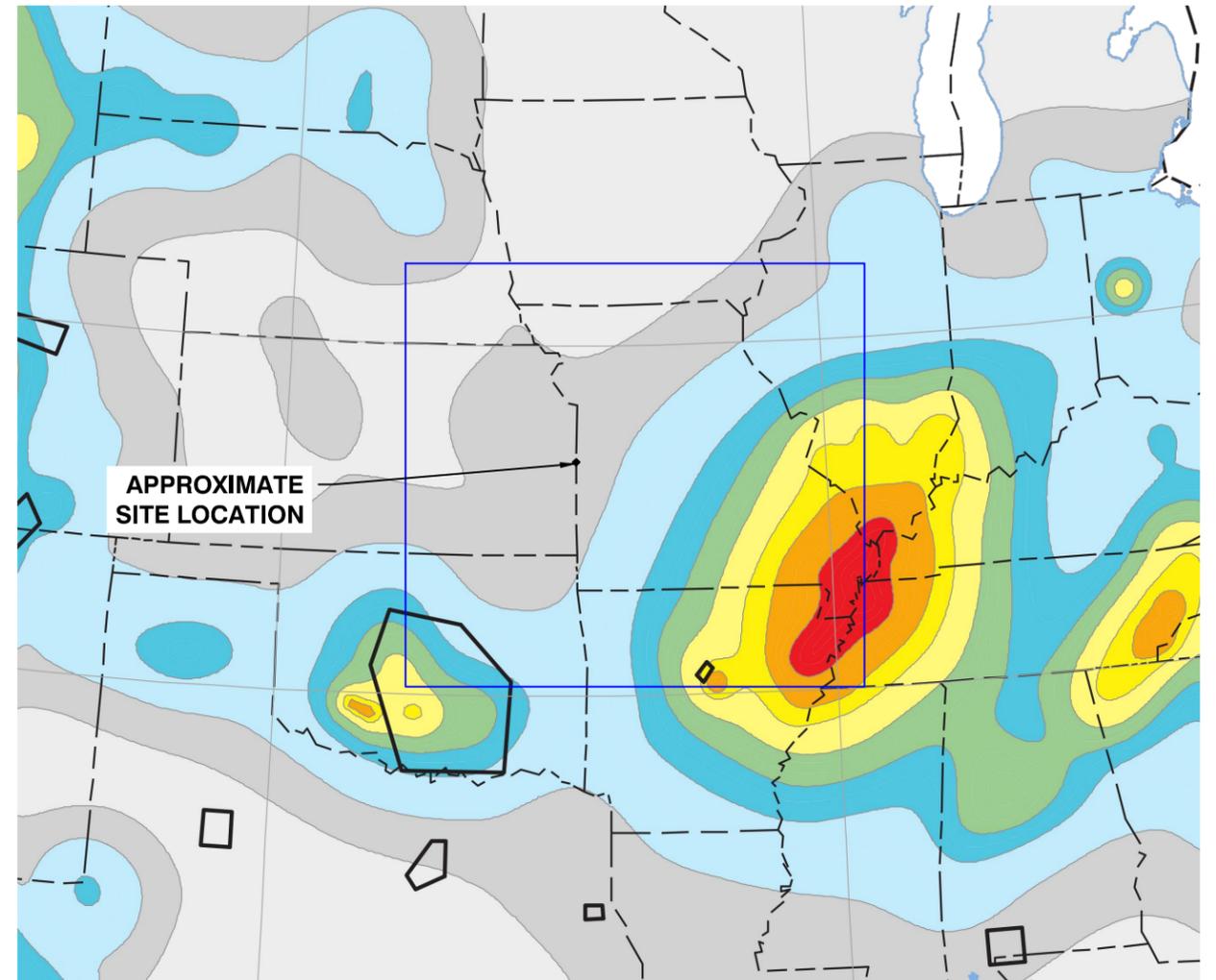
<b>FIGURE 3</b>			
<b>FAULT AREAS MAP</b>			
KCP&L LA CYGNE GENERATING STATION - UPPER AQC IMPOUNDMENT LA CYGNE, KANSAS			
<b>SCS ENGINEERS</b>			
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CHK. BY: DLD	DWN. BY: TGW	DSN. BY: TGW	PROJ. NO. 27218131.04
PROJ. MGR: DLD	DATE: 10/2018	CADD FILE: FIGURE_3_FAULT_AREAS.DWG	

**Two-percent probability of exceedance in 50 years map of peak ground acceleration**



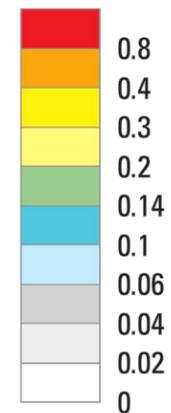
Source: USGS seismic impact zones map (2014) - <http://earthquake.usgs.gov/hazards/hazmaps/conterminous/index.php#2014>

0 200 MILES



**EXPLANATION**

Peak acceleration, expressed as a fraction of standard gravity (g)



Areas with suspected nontectonic earthquakes are not included.



**FIGURE 4  
HORIZONTAL ACCELERATION MAP**

KCP&L LA CYGNE GENERATING STATION - UPPER AQC IMPOUNDMENT  
LA CYGNE, KANSAS

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CHK. BY: DLD	DWN. BY: TGW	DSN. BY: TGW	PROJ. NO. 27218131.04
PROJ. MGR: DLD	DATE: 10/2018	CADD FILE: FIGURE_4_SEISMIC.DWG	

## APPENDIX A

### Placement Above the Uppermost Aquifer Supporting Information

- Portions of Detailed Hydrogeologic Site Characterization Report (AECOM 2017)

## 1.4 Site Setting Per Previous Studies

Information from previous Woodward-Clyde and URS investigations in 1979, 2005, and 2010 was incorporated with data from the current investigation to prepare this report. A summary of each of the prior investigations is provided below with the location of the monitoring wells and piezometers that were incorporated into the current investigation provided on **Figure A.5, Appendix A**. Historic boring logs, well construction logs, and a pump data table are included in **Appendices D.2, I.1, and I.2**, respectively.

Woodward-Clyde conducted a geologic and geotechnical investigation between June 22 and November 3, 1978 (Woodward-Clyde, 1978) to evaluate conditions relevant to the design and construction of the Site. The 1978 investigation included 147 test borings, 40 test pits, and 10 observation wells (OW-1 through OW-10). Representative soil and rock samples were obtained at select locations across the Site for laboratory analyses. Monitoring wells numbered MW-1 through MW-11 (originally MHC 1 through MHC-11) were installed between March and May 1979 and extended to depths of 34- to 40-ft. bgs (URS, 2002). These initial eleven wells are located around of the Site and north of the CCR Landfill, as presented on **Figure A.5, Appendix A**.

URS completed an investigation titled "Combined Supplemental Investigation and Groundwater Monitoring Report, KCP&L La Cygne Generating Station" (URS, 2005), in accordance with approval from KDHE in a letter dated April 28, 2004 for conducting supplemental field activities during the summer of 2004 to investigate groundwater conditions. This supplemental investigation consisted of installing, developing, and sampling three (3) monitoring wells (TW-1, TW-2, and TW-3) and four (4) piezometers (PZ-1S, PZ-1D, PZ-2S, and PZ-2D).

URS completed a geotechnical evaluation (URS, 2010) for the AQC Impoundments in 2010. This investigation consisted of nine exploratory borings; designated P-501 through P-509, advanced from the crest of the Site embankment through the embankment fill material into the underlying bedrock foundation. Upon completion of the drilling and sampling, a piezometer was installed at each location (P-501 to P-509).

### 1.4.1 Stratigraphy

A varying thickness of natural soil and some fill underlie the Station. In the Unified Soil Classification System (USCS), the natural soils vary from brown to gray, low to highly plastic clays (CH), silty clays (CL) and sandy clays (SC) with some discrete poorly graded sand beds or lenses (SP). The majority of the soils are residual deposits formed from the weathering of the underlying bedrock, although some alluvial or colluvial soils may be present. **These unconsolidated deposits vary from 1- to 31-ft. in thickness at the boring locations overlying bedrock comprised of primarily shale with some thin sandstone and limestone units.**

The shallow bedrock underlying the unconsolidated deposits is a stratigraphically and lithologically complex set of units that are transitional between the lower Pleasanton and upper Marmaton Groups of the Pennsylvanian System with a combined thickness of these units reported between 50 and 80 ft. The Pleasanton and Marmaton Groups are described as predominantly shale with some sandstone and limestone. Both shale and sandstone strata outcrop at various locations within the Station, generally along intermittent drainage ditches. Less frequently, discontinuous and localized limestone strata were observed at a few locations within the Station (Woodward-Clyde, 1978). Lateral facies changes and vertical gradational changes are common. Correlation of units between boreholes is somewhat limited since drilling methods used generally did not provide undisturbed samples resulting in shale units that are similar in appearance. However, the presence of a limestone unit in several borings potentially suggests the top of the Lenapah Formation (URS, 2005).

The elevation of top of bedrock surface underlying the Site is presented on **Figure A.7, Appendix A**. The contours shown were developed using site-specific boring information augmented with pre-plant topography as shown on **Figure A.4, Appendix A**. The bedrock surface reasonably parallels the pre-plant topography, which indicates the presence of a former west-flowing stream channel south of the Site located below the landfill and

During the 2015 Site Investigation, excluding SB series soil borings (SB-101, -102 and -103), all of the borings encountered shale bedrock at elevations ranging from 875 to 842-ft. Bedrock was cored in two of the borings (MW-702A and MW-707A).

### 3.2.4 Groundwater Conditions

As stated previously in Section 1.4.2, the uppermost water-bearing zone at the Site is typically encountered within the Holdenville Shale in the upper Marmaton Group. The majority of the groundwater monitoring wells are screened within this formation. The lithologic characteristics of this formation were described in the 2015 Site Investigation as a moderate to highly plastic shale with many calcareous and sandy zones and varying from brown and weathered to gray and unweathered.

The 2015 Site Investigation by AECOM confirmed the uppermost water-bearing zone at the Site is in a zone of saturated unweathered to highly weathered shale within the Holdenville Shale. For the purposes of the CCR Rule, this is the water-bearing unit that has been defined as the aquifer at the Site, as discussed in Section 3.3.2. During the groundwater monitoring sampling events, water levels from monitoring wells around the Site were measured by AECOM. These data were used to assess the magnitude and direction of the hydraulic gradient at the Site, and are presented on the potentiometric maps in **Appendix A.6**. The potentiometric maps show a generalized flow direction to the west across the Site toward La Cygne Lake with some west-southwest and west-northwest trends in the southeastern and northeastern portions of the Site, respectively. The water levels were monitored approximately every other month from June 2016 to June 2017 in each well at the Site. The seasonal variations of the groundwater elevations range from 842.04-ft. to 878.18-ft. from June 2016 to June 2017 at the Site. The table and chart of the water level measurements in the wells selected for the potentiometric maps over this period of time are included in **Table 2** and **Chart 1, Appendix C**. The calculations for hydraulic gradient for the water level readings from June 2016 to June 2017 are included in **Appendix C**. The average hydraulic gradient is approximately -0.013 measured from MW-11 to MW-705.

## 3.3 Site-Specific Technical Information (40 CFR 257.91(b))

The site-specific technical information required by 40 CFR 257.91(b) is summarized in the sections below. The information has been divided into sections based on the requirements of the CCR Rule in order to characterize the overlying geologic units, the uppermost aquifer, and the confining unit defining the lower boundary of the uppermost aquifer.

### 3.3.1 Overlying Geologic Units

The material overlying the aquifer at the Site consists of unconsolidated overburden materials and the unsaturated shale bedrock above the water-bearing zone within the Holdenville Shale.

The overburden is mostly stiff to very stiff, low to highly plastic clay and some surficial fill material at several locations across the Site. The thickness of the clay ranged from 5- to 17-ft. at the boring locations advanced in 2015, depending on the ground surface elevation and the thickness of the overlying fill. The thickness and composition of the unconsolidated deposits is generally consistent with the findings from previous investigations as discussed in Section 1.5.2. The vertical hydraulic conductivity of the clay unit was measured by conducting falling head permeability laboratory tests from representative samples collected within the clay unit from borings SB-101 and SB-102. The results of these tests indicated a hydraulic conductivity range of  $9.2 \times 10^{-09}$  to  $1.0 \times 10^{-06}$  cm/sec. None of the boring logs from the 2015 investigation noted encountering water during drilling. The unsaturated bedrock consists of the shale below the overburden and above the saturated zone. These rocks are dominated by shales but may also include limited sandstone and limestone units as described in Section 3.2.3 above.

## APPENDIX B

### Unstable Areas Supporting Information

## APPENDIX B.1

### Portions of Previous Reports

Portions of Detailed Hydrogeologic Site Characterization Report (AECOM, 2017)

Portions of Geotechnical Data for Bidders (WCC, 1978)

described as “deep, nearly level, and somewhat poorly drained”, and is found on uplands. Other soils present at the Station are consistent with uplands and side slopes, and are generally “deep, gently sloping, and moderately well-drained” (USDA, 1981).

The Eram silty clay soils are mapped in the central portion of the Station. These soils are described as moderately deep, nearly level, moderately well drained soils derived from clayey residuum weathered from sandstone and shale (USDA, 2016).

### 1.3.2.2 *Bedrock*

The bedrock near the Station consists of shale, limestone, sandstone and minor coal, assigned to the Middle to Upper Pennsylvanian Age, Marmaton and Pleasanton Groups (KGS, 1969). These units are relatively typical of the cyclic sedimentary rock deposits present in the Midwest. Thickness of individual beds varies from thin coal “smuts” to beds as thick as thirty feet, with lateral variations occurring in many locations (Jewett et al, 1968). A geologic column of the Marmaton and Pleasanton Groups is presented on **Figures B.2, Appendix B** and a surficial geologic map of the Site is presented on **Figure B.3, Appendix B**.

Stratigraphy is generally correlated from boring logs. Borings generally encounter shales of the Marmaton Group, which vary in thickness across the Station, with some limestone and sandstone units up to 11-ft. thick locally. The Holdenville Shale is the most commonly encountered bedrock, and is hard yellow-gray shale that can be up to 30-ft. thick. The Perry Farm Shale of the Lenapah Limestone Formation is also encountered in some areas, this is a hard gray shale that can be up to 10-ft. thick (Seevers, 1969).

### 1.3.2.3 *Structural Features*

The Station is located within the Forest City Basin, and the Bourbon Arch is the closest major structure, mapped approximately 40 miles to the southwest of the Station. This structure is described as “a WNW-ESE trending structural feature separating the Forest City Basin from the Cherokee Basin” (Jewett, 1951). **Figure B.4, Appendix B** presents the major geologic structures surrounding Linn County. The closest minor feature is the Mound City Dome, mapped approximately 15 miles south of the Station (Jewett, 1951).

Generally, there are no structural features that have been encountered or mapped at the Station, however it has been inferred that the Station may lie between the El Dorado Springs Fault and the Schell City – Rich Hill Anticline, both of which are NW-SE trending features that extend into Missouri (Gentile, 1976). If present, the El Dorado Springs Fault would be approximately 1-2 miles south of the Station, and the Schell City – Rich Hill Anticline would be approximately 1-2 miles north of the Station. A map of these features is presented as **Figure B.5, Appendix B**.

### 1.3.3 *Hydrogeologic Setting*

The Pennsylvanian bedrock units at the Station are not typically associated with a regional aquifer, whereas the deeper Ordovician units are included in the Western Interior Plains Aquifer. The Station is located near the northwestern edge of the Ozark Plateaus Aquifer system. Total Dissolved Solids within the Marmaton and Pleasanton Groups have been recorded at 632 and 222 ppm, respectively (Seevers, 1969).

Regional drinking water is typically obtained from groundwater production wells reportedly screened at depths from 30- to 200-ft. below ground surface (bgs), with shallower wells south of the Station installed in overburden, and deeper wells installed to the north, likely in bedrock of the Cherokee Group. The nearest domestic well to the Station, according to the KDHE Bureau of Water database, is located approximately one mile north by northeast, and is drilled to a depth of 170-ft. bgs into a thick shale, with a static water level of 35-ft. bgs (KGS, 2017).

**Figure A.3, Appendix A** presents a hydrologic map of the area. Major bodies of water, streams, and drainage courses are shown.

## 1.4 Site Setting Per Previous Studies

Information from previous Woodward-Clyde and URS investigations in 1979, 2005, and 2010 was incorporated with data from the current investigation to prepare this report. A summary of each of the prior investigations is provided below with the location of the monitoring wells and piezometers that were incorporated into the current investigation provided on **Figure A.5, Appendix A**. Historic boring logs, well construction logs, and a pump data table are included in **Appendices D.2, I.1, and I.2**, respectively.

Woodward-Clyde conducted a geologic and geotechnical investigation between June 22 and November 3, 1978 (Woodward-Clyde, 1978) to evaluate conditions relevant to the design and construction of the Site. The 1978 investigation included 147 test borings, 40 test pits, and 10 observation wells (OW-1 through OW-10). Representative soil and rock samples were obtained at select locations across the Site for laboratory analyses. Monitoring wells numbered MW-1 through MW-11 (originally MHC 1 through MHC-11) were installed between March and May 1979 and extended to depths of 34- to 40-ft. bgs (URS, 2002). These initial eleven wells are located around of the Site and north of the CCR Landfill, as presented on **Figure A.5, Appendix A**.

URS completed an investigation titled "Combined Supplemental Investigation and Groundwater Monitoring Report, KCP&L La Cygne Generating Station" (URS, 2005), in accordance with approval from KDHE in a letter dated April 28, 2004 for conducting supplemental field activities during the summer of 2004 to investigate groundwater conditions. This supplemental investigation consisted of installing, developing, and sampling three (3) monitoring wells (TW-1, TW-2, and TW-3) and four (4) piezometers (PZ-1S, PZ-1D, PZ-2S, and PZ-2D).

URS completed a geotechnical evaluation (URS, 2010) for the AQC Impoundments in 2010. This investigation consisted of nine exploratory borings; designated P-501 through P-509, advanced from the crest of the Site embankment through the embankment fill material into the underlying bedrock foundation. Upon completion of the drilling and sampling, a piezometer was installed at each location (P-501 to P-509).

### 1.4.1 Stratigraphy

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Page A.5

slotted pipe to a level approximately 2 ft above the topmost slot. Next, bentonite pellets were placed in the annulus forming an impermeable plug approximately 3 ft thick. The remaining length of the annulus was grouted using a cement-bentonite grout. A vented cap was placed on the top of the riser. The riser extends above ground approximately 3 ft at each location. All water level readings were referenced to the surveyed elevation at the top of the risers. A diagram showing typical observation well construction is included as Figure A-12.

#### REGIONAL GEOLOGY

The regional geology of the site was obtained from a review of the available geologic literature relevant to the site. A listing of the geologic literature reviewed is provided in the Bibliography.

The portion of eastern Kansas including the site lies in the Osage Cuestas section of the Osage Plains Physiographic Subprovince, a part of the Central Lowland Physiographic Province, Figure A-2. The Osage Cuestas are composed of a subparallel series of northeast-southwest trending, eastward facing escarpments, or cuestas. The valleys between the ridges are generally flat to gently rolling. The cuestas are invariably underlain by resistant Pennsylvanian age limestones; whereas, the intervening valleys are underlain by more easily eroded Pennsylvanian age shales, sandstones and siltstones. These rock units, composed of strata of the Pennsylvanian System of the Upper Paleozoic, normally dip to the west and northwest off the Ozark Dome in Missouri at approximately 20 ft per mile (less than 1 percent). They are designated sequentially

## APPENDIX B.2

### Geologic/Geomorphologic Features Documentation

Appendix D—Surface Sinkholes and Other Solution Features, Geologic History of Kansas

SEE STATE MAP AT  
END OF DOCUMENT  
FOR COUNTY CODES

## Geologic History of Kansas

[Prev Page--Structural Nomenclature](#) || [Next Page--Earthquakes in Kansas](#)

### Appendices, continued

#### Appendix D--Surface Sinkholes and Other Solution Features

From Merriam and Mann, 1957.

CA

**Ashland Basin (Ashland-Englewood Basin)**--The Ashland Basin in southern **Clark County**, southwestern Kansas, forms a large topographic depression. It is a coalescing sinkhole (Frye, 1942). The depression probably was caused by solution of Permian salt and gypsum, which occur less than 1,000 feet below the surface (Frye, 1950; Jewett, 1951; Smith, 1940).

This large sinkhole is as much as 12 miles wide and 500 feet below the general level of the High Plains (Schoewe, 1949). The walls are Permian redbeds capped by Ogallala (Pliocene). The basin is dissected and drained by Cimarron River. As much as 100 feet of late Pleistocene fill has been deposited in the depression. Frye (1950) dated its subsidence as mid-Pleistocene to late Pleistocene and believed that solution is directly related to faults that formed in Pleistocene time in the area.

CA

**Big Basin and Little Basin**--Two sinkholes in western **Clark County** are well known; the larger, Big Basin (Pl. 26A), is located just west of the smaller, Little Basin, which contains within its boundaries a picturesque and smaller sinkhole known as St. Jacob's Well (Pl. 26B).

Big Basin is situated in sec. 24 and 25, T. 32 S., R. 25 W. The sinkhole is subcircular and approximately one mile in diameter. The floor is relatively flat and 125 to 150 feet below the rim. Small depressions or sags, often retaining water, occur on the floor of the basin. The wall of the sinkhole is still essentially vertical although slightly dissected. Permian, Cretaceous, and Tertiary rocks crop out in the wall of the basin. Its formation probably occurred from several hundred to a few thousand years ago (Smith, 1940).

Little Basin is about one-third mile east of Big Basin. The floor is 35 feet below the rim level. Although Little Basin is shallower than Big Basin, the two sinkholes are believed to be the same age. St. Jacob's Well, a relatively recent sinkhole, is on the floor of Little Basin. The water which is retained in this smaller sinkhole forms an "oasis" in the semiarid country.

Both Big and Little Basins are believed to be due to solution of the underlying soluble Permian beds.

CR

**Cherokee County Sinkholes**--Sinkholes are observable where rocks of Mississippian age crop out in some areas of southeastern Kansas. All known sinkholes in **Cherokee County** are attributed to solution of the thick Mississippian limestone in post-Cherokee time, as well as recently. Howe (1956) reported that Pennsylvanian strata are commonly faulted in older sinkholes because the collapse of caverns in the Mississippian limestone occurred after consolidation of overlying sediments. Some faults have a throw of as much as 6 feet. Pyrite and marcasite are common along fault planes.

Nine recent sinkholes were described by Pierce and Courtier (1937). Old sinkholes dated as post-lower Cherokee are exposed in four of the recent sinkholes.

Two of the recent sinkholes subsided about 1905. One is located in the SW sec. 34, T. 32 S., R. 25 E.; the other is in the NW sec. 28, T. 33 S., R. 25 E. Both sinkholes are elliptical, 75 to 125 feet across, and 30 or more feet deep. Their walls are nearly vertical.

Two more sinkholes are in the W2 sec. 34, T. 32 S., R. 25 E. The earlier sinkhole was formed in 1911. A few years prior to 1911 a sag, about 14 feet in diameter, appeared in a corn field, but no water was retained in the depression. In 1911 a larger hole suddenly appeared in the area of the former sag. The wall was vertical and extended down 72 feet; water was present in the bottom. In 1933 the north rim sank approximately 20 feet. The second sinkhole in sec. 34 formed in 1922 and was a shallow depression about 10 feet in diameter.

Two other shallow sinkholes, about 10 feet in diameter, are located in the NE sec. 9, T. 32 S., R. 25 E.; one formed about 1921 and the other in 1929. Another sinkhole in the NE sec. 34, T. 32 S., R. 25 E., was reported to have formed in 1911 or 1912; it was filled prior to 1937, and no further data on it are available. In 1924 cracks appeared in the soil in the E2 sec. 9, T. 32 S., R. 25 E. The area of soil cracks subsided in 1929 to form a vertical-walled, elliptical sinkhole, 75 to 125 feet across and 30 feet deep.

HM

**Coolidge Sink**--On December 18, 1926, a hole suddenly appeared in the ground 15 miles south of Coolidge, **Hamilton County** (NE sec. 22, T. 25 S., R. 43 W.). On July 1, 1930, the sinkhole was reported to be about 60 feet in diameter and 40 feet deep (Bass, 1931). It was circular with a steep and undercut wall; the floor sloped at a low angle toward the center of the depression. Three sets of crevices encircled the structure.

By August 8, 1930, the sinkhole had enlarged to a diameter of 104 feet and increased in depth to 68 feet (Bass, 1931). The material in which the sinkhole was formed was homogeneous silt; no stratified rock could be seen in the hole. Smith reported that in 1940 water filled the hole to within 10 feet of the surface and that the wall had slumped so that no overhang was visible. The depression had increased in size to 150 by 200 feet in 1941 and had elongated from its original circular shape, engulfing a nearby country road. It also had filled with water to within 15 feet of the rim (McLaughlin, 1943).

Bass attributed the sinkhole to solution and formation of a cavern with subsequent collapse of the roof. He reported that bedrock dips at 5' toward the sinkhole, indicating that the entire area of recent subsidence may be part of a larger and older sinkhole. Landes (1931) studied the area and determined that Graneros Shale was exposed below the rim. From this, Landes decided that the original cavern must have been formed in either salt or gypsum in the Permian section. Smith found that this sinkhole is but one of a linear series and suggested that they occur along a post-Ogallala fault.

ME

**Meade Salt Sink**--Sudden sinking of a circular area 150 to 200 feet across took place sometime between the 3rd and 18th of March 1879, in **Meade County**, 1 1/2 miles southeast of Meade. The sinkhole is on the east side of Crooked Creek, just east of the Crooked Creek Fault. The Great Salt Well, as it was called at the time of formation, engulfed a portion of the "Jones and Plummer Trail," an often-used wagon road and cattle trail (Cragin, 1884). Mudge (1879) stated that by the 18th of March it was 60 feet deep and had a circumference of 610 feet; it was nearly circular with a perpendicular wall. Saline water filled the hole to within 17 feet of the land

surface. The depth of the water ranged from 15 to 27 feet at the edge to 42 feet at the center. Water finally rose to within 14 feet of the ground surface.

Sod cracks formed on the rim around the sinkhole. They were 5 to 15 feet deep and 1 to 10 inches wide. The more distant cracks were 126 feet from the hole.

Mudge (1879) stated that 1 bushel of salt was recovered for every 43 gallons of water, about a 7 percent concentration. At one time salt was produced commercially from the well.

Presently the sinkhole is filling with sediment. In the last few years the sinkhole has been completely dry (W. H. Schoewe, personal communication).

Mudge (1879) thought that a cavern had been formed in the Dakota Formation, softer material having been washed out by subterranean water, causing subsequent caving. Johnson (1901), Smith (1940), and Frye (1942) suggested that the cavern formed in underlying Permian salt beds and that overlying rocks collapsed. Frye (1942) pointed out that the shallowest soluble rock is in Permian redbeds several hundred feet below the present water table. He believed that Pliocene faulting provided the necessary openings for water to have access to soluble beds in the Permian and the faulting therefore controlled development of solution caverns; the period of sinkhole formation thus has been limited to post-Pliocene time.

**MC Mitchell County Sink**--An unusual type of sinkhole developed in **Mitchell County** in 1927. Subsidence in the form of a trench in loess, 200 feet long, 75 feet wide, and 18 feet deep, necessitated moving a farm house. Landes (1932) reported that flat discoidal pebbles of the Fort Hays Limestone, derived from nearby outcrops, are found in loess at the level of the trench floor. He believed that solution of limestone pebbles formed a small cave in the loess, that erosion by circulating ground water enlarged the cave, and that the result was collapse of the overlying loess and formation of a sinkhole. A second collapsed cave 40 feet long was formed in 1931 at right angles to the main trench.

**ME Jones Ranch Sink**--The Jones Ranch Sink, 8 miles southeast of Meade, forms a large topographic depression in **Meade County** (T. 32 and 33 S., R. 27 W.). The sinkhole is subcircular, 3 miles in diameter, and controls a centripetal drainage pattern. It is dissected and partly filled with sediment. Exposed bedrock dips slightly into the sinkhole. Its origin is attributed to solution and collapse (Smith, 1940; Frye, 1942; Jewett, 1951). From the molluscan fauna which it contains, Frye and Leonard (1952) dated the fill as early Wisconsinan. They pointed out that isolated features such as this are dated either by fossils or by intersection of lines of dissection.

**WA Old Maid's Pool**--This sinkhole is northwest of Sharon Springs in **Wallace County** (sec. 30, T. 12 S., R. 40 W.). Moore (1926b) described it as 80 feet in maximum depth and three-eighths of a mile in diameter. It holds a small lake 300 feet wide, although in 1962 it was dry. The hole is circular, and the wall is moderately dissected. Pierre Shale is exposed on the south side of the basin (Elias, 1931).

**BU Potwin Sink**--A sinkhole which occurred suddenly near Potwin has been described by Gordon (1938). The hole, located in the SE sec. 24, T. 24 S., R. 3 E., **Butler County**, was formed on the afternoon of September 22, 1937. It was 90 feet by 150 feet, elongated in an east-west direction, and about 45 feet deep. Water filled the hole to within 15 feet of the rim. The unstratified loam of the wall of the sinkhole was perpendicular. It was estimated that approximately 500,000 cubic feet of material was involved in the subsidence. The rim was estimated to be about 35 feet below base of the Herington Limestone. Gordon believed that solution had taken place in the Fort Riley Limestone about 75 feet below base of the sinkhole and that sudden collapse of the cavern roof resulted in the sinkhole.

Two older partly filled sinkholes are found in the vicinity of the Potwin Sink.

**WA Smoky Basin Cave-in**--A sudden subsidence which took place near Smoky Hill River about 5 miles east of Sharon Springs on March 9, 1926, attracted nationwide attention. The sinkhole (sec. 33 and 34, T. 13 S., R. 39 W., **Wallace County**) had a diameter of about 50 feet. By March 11 it had increased in size to about 125 by 250 feet, giving it an irregular elliptical shape (Moore, 1926b). By April 13 its dimensions had increased to 150 by 290 feet, and still later to about 250 feet north-south by 350 feet east-west. The wall was vertical down to the water level, 165 to 170 feet below the lip. Water was 50 feet deep in the center of the hole. Moore (1926a) estimated the total depth of the sinkhole to be 300 to 350 feet and the amount of material involved to be approximately 1 1/2 million cubic feet. Pierre Shale is exposed in the wall of the sinkhole, and the Niobrara Formation is exposed a short distance east of the area.

Moore (1926a) suggested that because of its size and the large amount of material involved, the sinkhole was due to roof collapse into a cavity of considerable size in chalk in the upper part of the Niobrara. Chalk was dissolved by eastward-moving ground water, which entered the formation at the outcrop farther west.

Russell (1929) discarded Moore's idea of solution of the Niobrara and postulated instead that the sinkhole was the indirect result of structural deformation. He thought there was too much shale in the Niobrara to be dissolved for cavern formation, and he found no evidence that the formation carried water. The clue to the origin of the sinkhole, he believed, lies in the structure of the region, because in nearby counties there are many faults which were formed during pre-Ogallala and post-Ogallala deformation. He believed that cavities, strong enough to resist pressures for a long time, occurred along faults, ultimately collapsing as did the Smoky Basin Cave-in. Many previously formed sinkholes probably have been obliterated by erosion and deposition so that only recent ones are evident. Russell reported a fault having a throw of 50 feet in the north wall of the cave-in.

Elias (1930) was of the opinion that the theory of collapse or subsidence along fault voids is unsound. He suggested that the fault may have aided indirectly in the cave-in by permitting surface water to descend underground and gain access to underlying chalk, causing subsequent solution in the Niobrara Formation.

**RL Flint Hills Sinkholes**--Hay (1896) described numerous sinkholes on upland areas of the Flint Hills in the Fort Riley Military Reservation, **Riley County**. Although not large, deep, nor spectacular, they are described here because they are representative of the common upland type in Kansas.

These sinkholes range in diameter from 30 to 50 feet and, in depth from 8 to 10 feet. All are roughly circular or oval in shape. Hay counted as many as 42 individual sinkholes in 1 square mile. Most individual sinkholes do not retain water. One of the larger sinkholes in the area contains five smaller ones in its floor. Upland sinkholes on the Fort Riley Reservation have been formed by solution and subsidence of the Fort Riley Limestone, which lies near the surface in the region. The Fort Riley is readily soluble and exhibits well-defined jointing.

Similar sinkholes have been reported in Wabaunsee County by Savage (1881) and in Morris County by W. R. Atkinson (personal

communication), Hay (1896), and Schoewe (1949). Small, circular, shallow sinkholes due to solution of the Fort Riley Limestone also occur in Cowley County (Bass, 1929) and Butler County (Fath, 1921).

**Other Solution Features**--Commonly associated with sinkhole development in Kansas are caves, natural bridges, underground drainage, and soil cracks. These related features are prominent mainly in the western part of the state. For the most part they are small and do not form a major part of the physiography.

Lee and Payne (1944) described an interesting occurrence of cave deposits in Mississippian rocks found in the subsurface of the McLouth gas and oil field, in Jefferson and Leavenworth counties. Pennsylvanian deposits occur in the caves, which were found at depths of as much as 150 feet below the top of the Mississippian.

Davis (1955) described in detail three small caves in the Stanton Limestone (Pennsylvanian) in Wilson and Montgomery counties.

Caves, presumably in the Fort Riley Limestone (Permian), have been mentioned by Savage (1881) as occurring in Wabaunsee County. Stalactites and stalagmites occur in the caves. W. R. Atkinson (personal communication) reported the occurrence of caves in the Fort Riley Limestone in southwestern Morris County. Caves in this part of the geologic section are usually low and narrow but long.

Solution of gypsum in the Blaine Formation (Permian) has resulted in many small sinks, caves, and natural bridges in Barber and Comanche counties. Caves in this area and adjoining portion of Oklahoma are commonly called "The Bat Caves." These caverns have been described in detail by Twente (1955). Grimsley and Bailey (1899) described a gypsum cave on Cave Creek, 4 miles west of Evansville, known as Big Gypsum cave. A stream entered from the west of the 100-foot cave and left by an east opening.

The largest and best known natural bridge in Kansas spanned Bear Creek at a point 7 miles south of Sun City in Barber County (Pl. 26C). The bridge, 12 feet high, 55 feet long, and 35 feet wide, collapsed late in 1961 (D. J. Malone, personal communication). Many other small natural bridges occur in this part of the state, but they are not as well known as the Sun City Natural Bridge. Such bridges have also formed by solution of Permian gypsum.

The absence of surface drainage courses may indicate subterranean water courses in soluble rocks. In the Bird City area in Cheyenne County, there is an absence of drainage channels. In western Kansas, White Woman Creek offers an excellent example of subsurface water drainage. The stream enters the state in Greeley County and flows east across Wichita County into western Scott County, where the surface water course disappears. No re-entry to the surface is known. The point at which the stream disappears is a short distance west of the Modoc Basin.

McLaughlin (1946) mentioned the area of the Bear Creek depression, where Bear Creek crosses the northwestern corner of Grant County. In places, drainage consists of a series of sinkholes and short intermittent streams, indicating underground water channels. In southern Kearny County, surface expression of the Bear Creek drainage ends abruptly. Many other small streams, especially in Wichita, Scott, Kearny, Finney, Grant, and Haskell counties, flow for short distances on the surface and then disappear underground.

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County key for counties discussed above. Note that Linn County was not discussed above.

[Kansas Geological Survey](#), Geologic History of Kansas

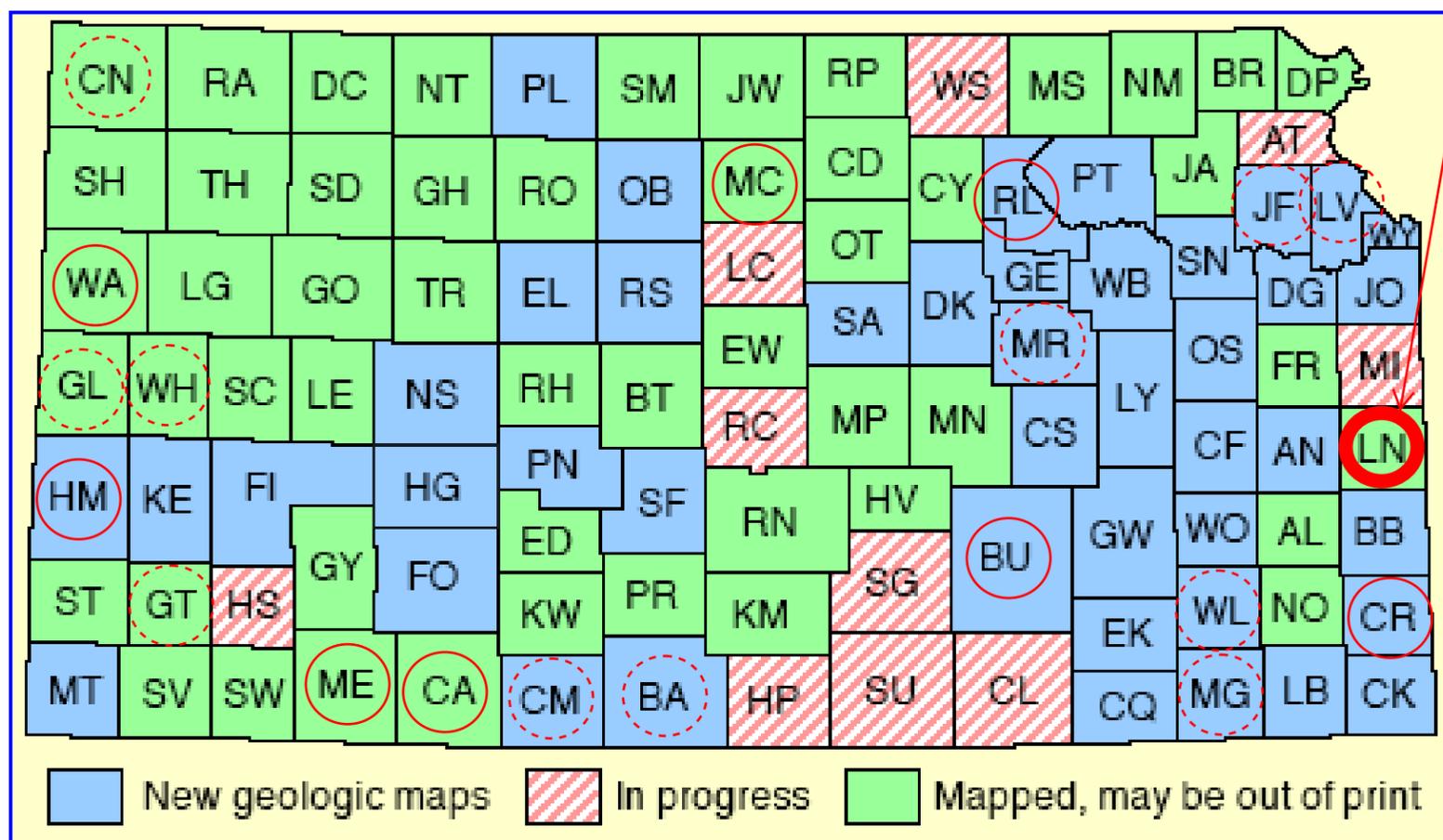
Comments to [webadmin@kgs.ku.edu](mailto:webadmin@kgs.ku.edu)

Web version April 2006. Original publication date Dec. 1963.

URL=[http://www.kgs.ku.edu/Publications/Bulletins/162/10\\_app\\_d.html](http://www.kgs.ku.edu/Publications/Bulletins/162/10_app_d.html)

LA CYGNE STATION

## County Geologic Maps



## APPENDIX B.3

### Human-made Features or Events Documentation

Ebasco Cross Sections from 1969

KGS Publication 114, Part 2, Coal Resources of the Marmaton Group in Eastern Kansas

AN 8980 G-549

BORING LOCATIONS

NO.	N-S	E-W	REMARKS
1	2010N	O	NOT TAKEN
2	1610N	O	NOT TAKEN
3	1210N	10E	
4	810N	O	NOT TAKEN
5	410N	10E	
6	544N	570W	
7	544N	430W	
8	904N	980W	
9	904N	790W	
10	904N	650W	
11	834N	600W	
12	974N	600W	
13	1054N	1080W	
14	1224N	1140W	
15	2250N	520W	
16	2250N	1400E	
17	2050N	1400E	
18	510N	1375E	
19	510N	1425E	
20	1094N	600W	
21	714N	600W	
22	974N	880W	
23	834N	880W	
24	974N	720W	
25	834N	720W	
26	310N	500W	
27	110N	510W	
28	190S	550W	NOT SHOWN ON DWG.
29	490S	650W	" " " "
30	790S	750W	" " " "
31	1710N	1400E	
50	2530N	180W	
51	2110N	180W	
52	2530N	220E	
53	2310N	220E	
54	2530N	620E	
55	2110N	620E	
56	2530N	1020E	
57	2310N	820E	
58	2110N	820E	
A	USED FOR WATER	LOCATION UNKNOWN	
B	LEVEL DETERM-	" " " "	
C	NATION	" " " "	

NOTES:

THIS DRAWING HAS BEEN PREPARED FOR THE USE OF DESIGN PERSONNEL OF EBASCO SERVICES INCORPORATED ONLY. INFORMATION CONTAINED HEREON IS AN INTERPRETATION OF THE ORIGINAL LOGS OF THE BORINGS FOR DESIGN PURPOSES ONLY AND MAY NOT BE COMPLETE DATA NOT PERTINENT TO DESIGN PURPOSES OR AVAILABLE IN OTHER REPORTS OR DATA SHEETS MAY NOT BE INDICATED HEREON. ANY PERSON USING THIS DRAWING FOR ANY PURPOSE OTHER THAN THE ABOVE SHOULD REQUEST TO EXAMINE AND REFER TO THE ORIGINAL LOGS AND DATA SHEETS.

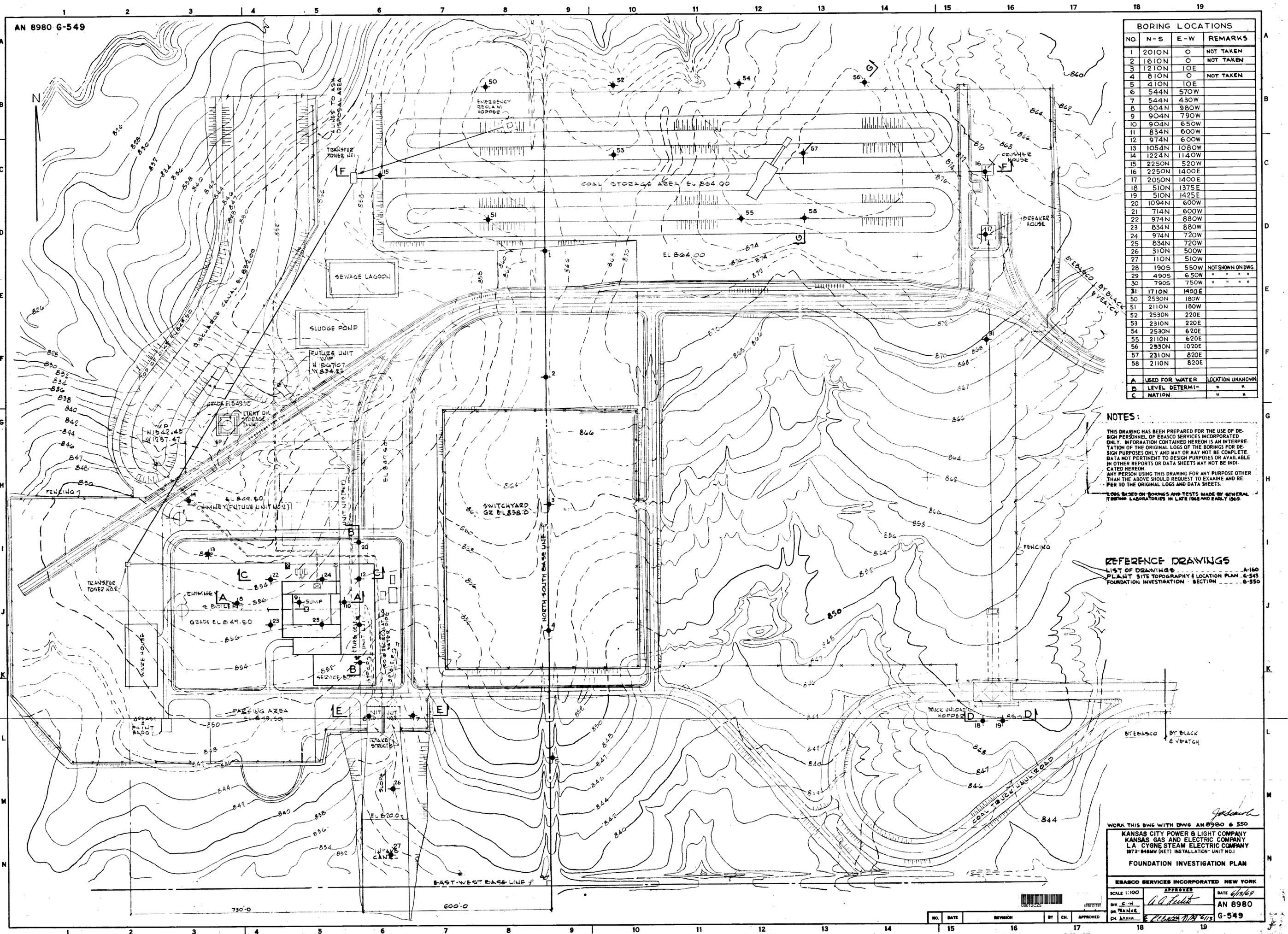
LOGS BASED ON BORINGS AND TESTS MADE BY GENERAL TESTING LABORATORIES IN LATE 1968 AND EARLY 1969.

REFERENCE DRAWINGS

LIST OF DRAWINGS: A-160 PLANT SITE TOPOGRAPHY & LOCATION PLAN G-545 FOUNDATION INVESTIGATION SECTION G-550

WORK THIS DWG WITH DWG AN 8980 G-550  
 KANSAS CITY POWER & LIGHT COMPANY  
 KANSAS GAS AND ELECTRIC COMPANY  
 L.A. CYGNE STEAM ELECTRIC COMPANY  
 1873-848W (NET) INSTALLATION UNIT NO. 1  
**FOUNDATION INVESTIGATION PLAN**

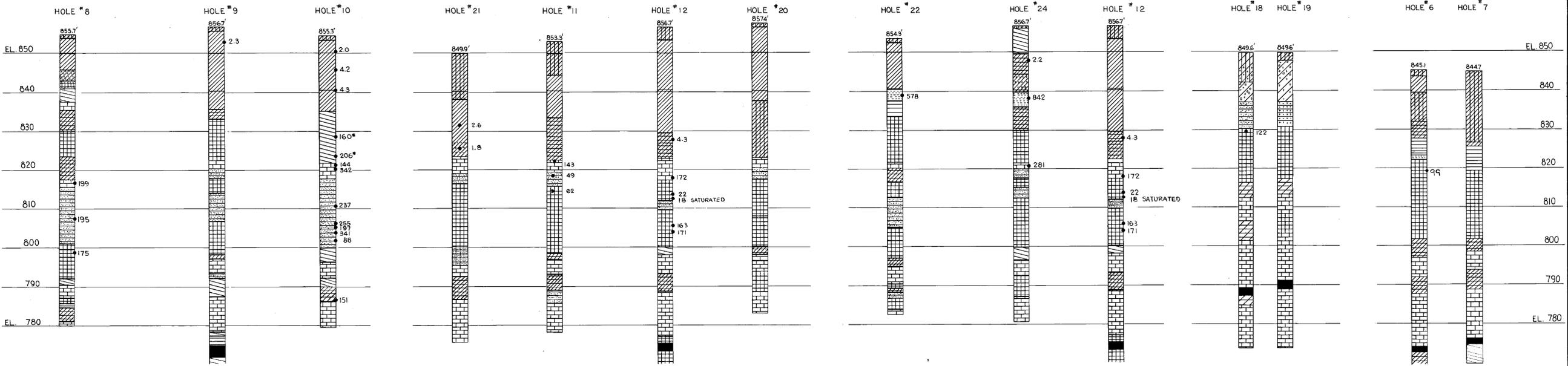
EBASCO SERVICES INCORPORATED NEW YORK  
 SCALE 1:100  
 DATE 4/16/69  
 DWG. E. H. [Signature]  
 DR. TRAINER  
 CH. MANAGER [Signature]  
 AN 8980  
 G-549



NO.	DATE	REVISION	BY	CHK.	APPROVED

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19

AN 8980 G-550



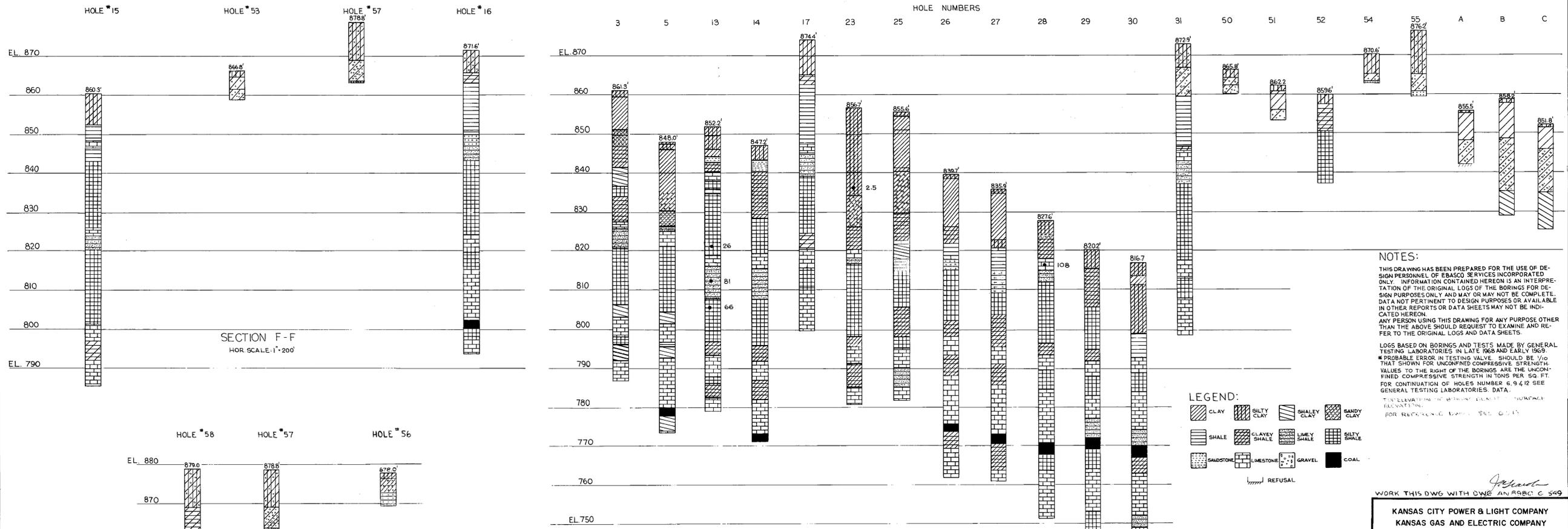
SECTION A-A  
HOR. SCALE: 1"=50'

SECTION B-B  
HOR. SCALE: 1"=50'

SECTION C-C  
HOR. SCALE: 1"=50'

SECTION D-D  
HOR. SCALE: 1"=100'

SECTION E-E  
HOR. SCALE: 1"=100'



SECTION F-F  
HOR. SCALE: 1"=200'

SECTION G-G  
HOR. SCALE: 1"=100'

- LEGEND:
- CLAY
  - SILTY CLAY
  - SHALEY CLAY
  - SANDY CLAY
  - SHALE
  - CLAYEY SHALE
  - LIMY SHALE
  - SILTY SHALE
  - SANDSTONE
  - LIMESTONE
  - GRAVEL
  - COAL
  - REFUSAL

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ANY PERSON USING THIS DRAWING FOR ANY PURPOSE OTHER THAN THE ABOVE SHOULD REQUEST TO EXAMINE AND REFER TO THE ORIGINAL LOGS AND DATA SHEETS.  
LOGS BASED ON BORINGS AND TESTS MADE BY GENERAL TESTING LABORATORIES IN LATE 1968 AND EARLY 1969.  
\*PROBABLE ERROR IN TESTING VALVE SHOULD BE 1/10 THAT SHOWN FOR UNCONFINED COMPRESSIVE STRENGTH. VALUES TO THE RIGHT OF THE BORINGS ARE THE UNCONFINED COMPRESSIVE STRENGTH IN TONS PER SQ. FT. FOR CONTINUATION OF HOLES NUMBER 6, 9, 4, 12 SEE GENERAL TESTING LABORATORIES' DATA.  
ELEVATION OF SURFACE OF GROUND AT SURFACE OF BORING IS INDICATED BY A DOTTED LINE.  
FOR REFERENCE SEE SECTION G-549

WORK THIS DWG WITH DWG AN 8980 C 549

KANSAS CITY POWER & LIGHT COMPANY  
KANSAS GAS AND ELECTRIC COMPANY  
LA CYGNE STEAM ELECTRIC STATION  
1973-848 MW (NET) INSTALLATION - UNIT NO. 1  
FOUNDATION INVESTIGATION - SECTIONS

EBASCO SERVICES INCORPORATED NEW YORK

SCALE: AS SHOWN

APPROVED: [Signature]

DATE: 1/1/69

DR. C. H. [Signature]

CH. [Signature]

AN 8980  
G-550

NO.	DATE	REVISION	BY	CH.	APPROVED



Bradley-Vernon mine shaft was sunk. According to Mudge (Gray, 1878, p. 87) coal was mined at Boicourt as early as 1872, for from 1873 to 1877, a total of 167 cars of coal of 18 tons capacity each was shipped from Boicourt on the Missouri River, Fort Scott, and Gulf Railroad. Gray (1875, p. 319) mentions the sinking of a mine shaft to a depth of 90 feet and encountering a coal seam measuring 40 inches in thickness. Traces of these early mines are no longer to be seen unless the mines were located at the sites of the more recent mines of the 1890's and 1910's.

**Production**--Except for the reference (Gray, 1878, p. 87) to the 167 cars of coal shipped from Boicourt between 1873 and 1877 the only coal production figures published by the Kansas coal mine inspectors are those shown in Table 19. A minimum production, therefore, of 31,698 tons plus 3,006 tons shipped between 1873-1877 or 34,704 tons of coal may be assigned to the Boicourt mining district.

**Table 19**--Coal production in the Boicourt coal-mining district, Linn County, Kansas.

Year	Bradley-Vernon mine, tons	Boicourt Coal Co. mine, tons	Total
1893	18,873		18,873
1894			
1895	11,037		11,037
1906		1,150	1,150
1913		208	208
1914		215	215
1915		215	215
<b>Total</b>	<b>29,910</b>	<b>1,788</b>	<b>31,698</b>

**Reserves**--The measured coal reserve area of the Boicourt coal-mining district comprises 1.1 square miles. On the basis of 34 inches as the average thickness of the coal seam, 3,590,400 tons of coal originally underlay the district. Subtracting from this amount approximately 35,000 tons known to have been mined leaves thus a measured reserve of 3,555,400 tons of coal (Table 14). As for the other mining districts in Linn County, the indicated and inferred coal reserves are best considered in relation to the county as a whole.

### North Sugar Creek Coal-mining District

The North Sugar Creek coal-mining district is along North Sugar Creek in the northeastern part of Linn County 5 to 8 miles east and southeast of LaCygne. The mines are all in R. 25 E., 10 mines being in T. 19 S., and 16 in T. 20 S. All the mines except 2 small strip pits are underground mines, none of which was in operation in 1953. As in the other mining districts, some of the mines changed ownership and consequently changed names. In most cases it is impossible to tell from the published records which mines operated under more than one name. Reports of the State coal mine inspectors record 33 mines, although field evidence for only 26 mines is to be found in the district. Data pertaining to the mines of this district are given in Table 20.

**Table 20**--Mulberry coal mines, North Sugar Creek coal-mining district, Linn County, Kansas.

Name	Location	Depth to coal, feet	Thickness of coal, inches	Type of mine
Jarred	NW 35-19-25E	86		Underground
Henry McNabb	NW cor. SW 35-19-25E	72		Underground
Vantyle	NW SW SW 35-19-25E	90	36-42	Underground
Vantyle	SW cor. SW 35-19-25E	65	36-42	Underground
Max Cerise Cox	NW 34-19-25E			Underground
C. Good	SW cor. 34-19-25E	40		Underground
Pinkert-Hall	SE SW 34-19-25E			Underground
	N. line SE 34-19-25E			Underground
	N. line SE 34-19-25E			Underground
	NE cor. SW 33-19-25E			Underground
Ben Good	NW. cor. 2-20-25E			Underground
Hall-Charlie Good	SE cor. NW NW 2-20-25E	50	48	Underground
King	NE cor. 3-20-25E	60-65		Underground
Berry-Vantyle	NE cor. NW 3-20-25E			Underground
	NW cor. 3-20-25E			Underground
	E. line NW 2-20-25E			Underground
	SE cor. NW SW 8-20-25E			Underground
	NE cor. NE SW 8-20-25E			Underground
Schirard	SE cor. NE SW 8-20-25E			Underground
	S. line NE SW 8-20-25E			Underground
	NE cor. SW SW 8-20-25E			Underground
	NW cor. NW SE 8-20-25E			Underground
	NE cor. NW SE 8-20-25E			Underground
Rucker	SW 9-20-25E	15		Strip
Gage	NW cor. 16-20-25E	15		Underground
	W. line SW 16-20-25E			Underground

The coal lies at depths ranging from 15 to 90 feet and is 3 to 4 feet thick. The two small strip mines reported are farmyard pits no more than 200 feet long and less than 50 feet wide with overburdens of about 15 feet. Because of the location close to drainageways, the coal is of poor quality and consequently was not exploited to any appreciable extent. The last mine in the district to operate was the Shirley slope in the SE cor. NE SW sec. 18, T. 22 S., R. 25 E. This mine ceased mining coal at the close of the 1951 mining season. All the mines served local trade.